

# Strengthening concrete structures with FRP laminates

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EUROCODES

EN 1992

Design  
of concrete  
structures

2<sup>nd</sup> generation of Eurocode 2 on concrete structures

Madrid, October 17<sup>th</sup>, 2023

**ACHE**  
Asociación Española de  
Ingeniería Estructural

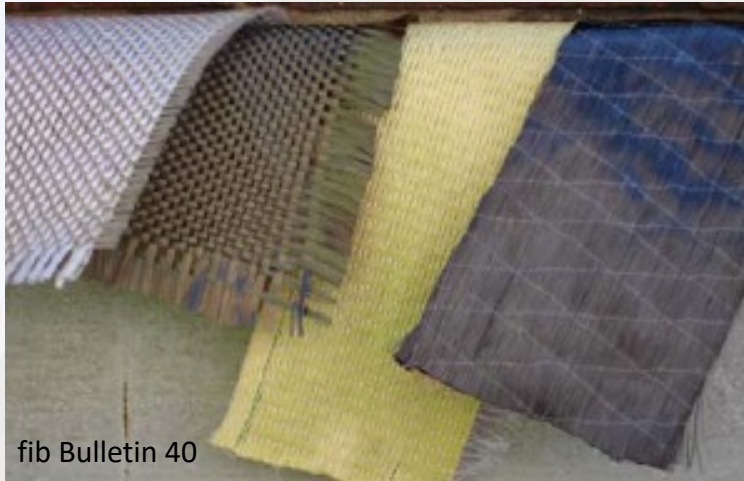
**HA**  
HORMIGÓN  
y ACERO

## Contents

1. Introduction
2. Basis of design, materials and structural analysis
3. Ultimate Limit States (ULS)
4. Serviceability Limit States (SLS)
5. Bond and anchorage for adhesively bonded CFRP systems
6. Conclusions

# 1. INTRODUCTION

**FIBRE REINFORCED POLYMER (FRP):** Composite material formed by a polymeric matrix (resin) reinforced with continuous fibres



Fibres:  
G (glass), C (carbon), B (basalt), A (aramid)

+



Matrix:  
Thermosetting; Thermoplastic

=

GFRP  
CFRP  
BFRP  
AFRP



# 1. INTRODUCTION

## STRENGTHENING STRUCTURES with FIBRE REINFORCED POLYMER (FRP) LAMINATES

to restore or increase their load bearing capacity



FRPs related to other industries



FRPs introduced in **construction sector** to overcome the drawbacks of steel bonded plates (corrosion and weight)



# 1. INTRODUCTION

## STRENGTHENING STRUCTURES by ADHESIVELY BONDED REINFORCEMENT (ABR) with **CARBON FIBRE REINFORCED POLYMER (CFRP)** LAMINATES

Included in **ANNEX J** (informative) of **EUROCODE 2**

- 2 possible CONFIGURATIONS



Externally bonded reinforcement (EBR)

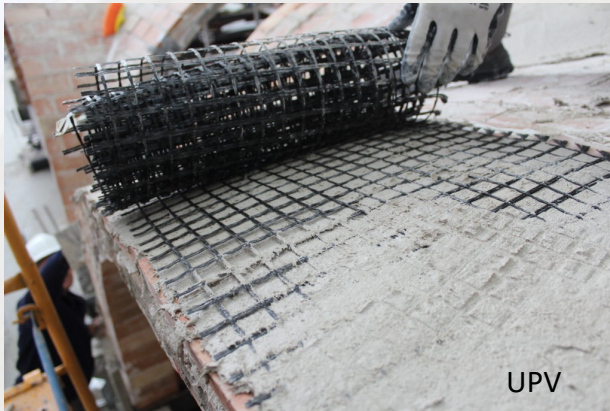


Near surface mounted (NSM) reinforcement

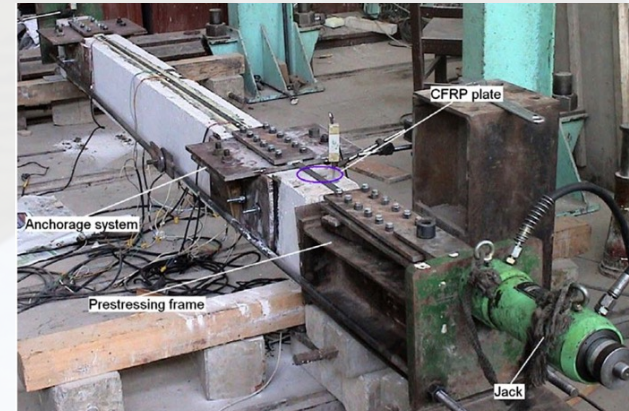
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**NOT** Included in **ANNEX J** (informative) of **EUROCODE 2**



Textile Reinforced Mortar (TRM)

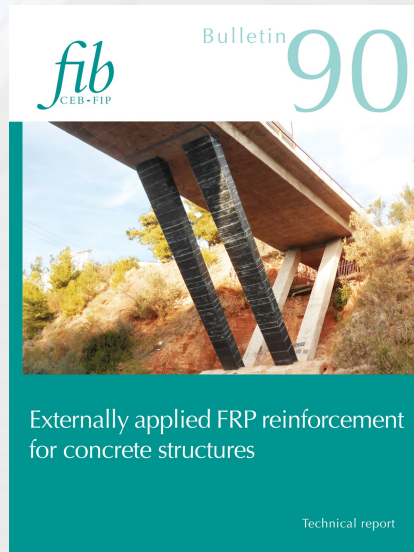


Prestressed adhesively bonded reinforcement

# 1. INTRODUCTION

## STRENGTHENING STRUCTURES by ADHESIVELY BONDED REINFORCEMENT (ABR) with **CARBON FIBRE REINFORCED POLYMER (CFRP)** LAMINATES

Existing European guidelines

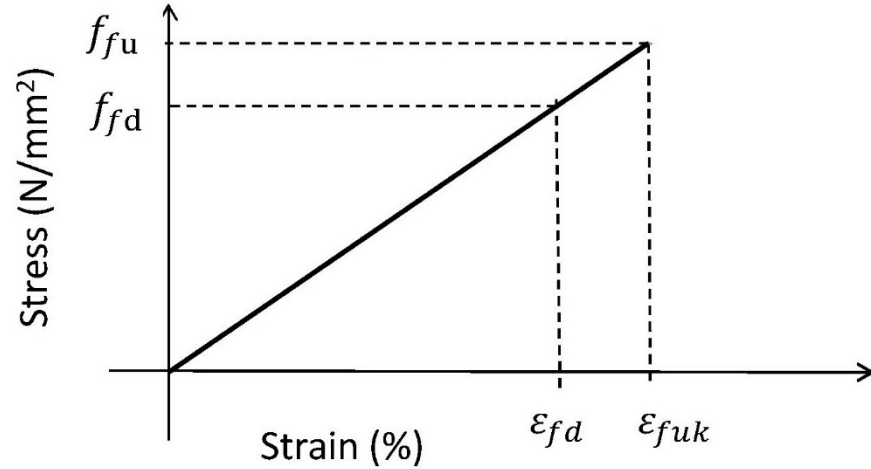
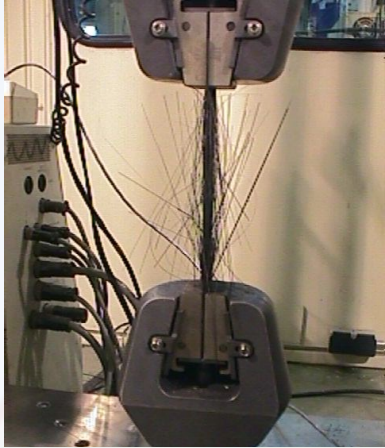


- DAFStb (2013) 
- CNR-DT 200 R1/2013 (2013) 
- TR-55 (2012) 
- AFGC (2011) 
- SIA (2004) 
- GRECO (2013) 

Additional background to Annex J

## 2. BASIS OF DESIGN, MATERIALS AND STRUCTURAL ANALYSIS

- **Linear elastic up to failure**



Design situation	Tensile strength		Bond strength
	CFRP strips and bars	In-situ lay-up CF sheets	Failure in concrete or adhesive
Designation	$\gamma_f$		$\gamma_{BA}$
Persistent and transient	1.30	1.40	1.50
Accidental	1.10	1.15	1.15
Serviceability	1.00	1.00	1.00
Fatigue	1.30	1.40	1.50

$$f_{fd} = \frac{\eta_f \cdot f_{fuk}}{\gamma_f}$$

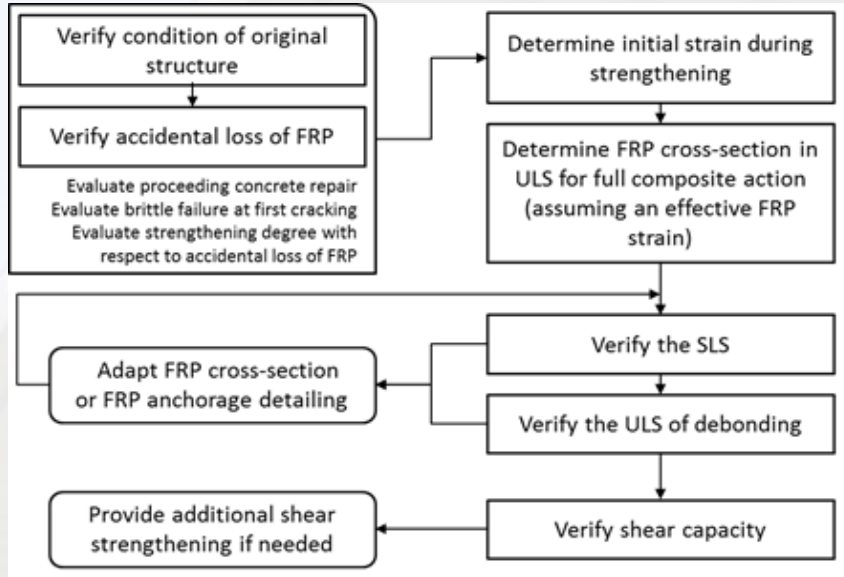
$\eta_f$  Reduction factor for relevant exposure conditions

Members strengthened with ABR should not be analysed using linear elastic analysis with limited redistribution or plastic analysis



### 3. ULTIMATE LIMIT STATES

#### Bending with or without axial forces



Design of flexural strengthening system by applying equilibrium + compatibility

**CHECK FRP DEBONDING!!!**



### 3. ULTIMATE LIMIT STATES

#### Confinement

FRPs apply an ever-increasing confinement pressure to the concrete core.

Ultimate capacity is governed by tensile failure of FRP (lower than standard tensile testing of FRP coupons).

based on **Lam and Teng (2003)**



For circular columns:

$$\Delta f_{cd} = 0 \quad \text{for } \frac{t_f \cdot f_{fud}}{D \cdot f_{cd}} < 0.07$$

$$\Delta f_{cd} = k_{cc} \cdot \frac{t_f}{D} \cdot f_{fud} \quad \text{for } \frac{t_f \cdot f_{fud}}{D \cdot f_{cd}} \geq 0.07$$

For rectangular columns:

$$\Delta f_{cd} = 0 \quad \text{for } \left(\frac{b}{h}\right)^2 k_e \frac{t_f \cdot k_r \cdot f_{fud}}{D_{eq} \cdot f_{cd}} < 0.07$$

$$\Delta f_{cd} = k_{cc} \cdot \left(\frac{b}{h}\right)^2 \cdot k_e \cdot \frac{t_f}{D_{eq}} \cdot k_r \cdot f_{fud} \quad \text{for } \left(\frac{b}{h}\right)^2 k_e \frac{t_f \cdot k_r \cdot f_{fud}}{D \cdot f_{cd}} \geq 0.07$$



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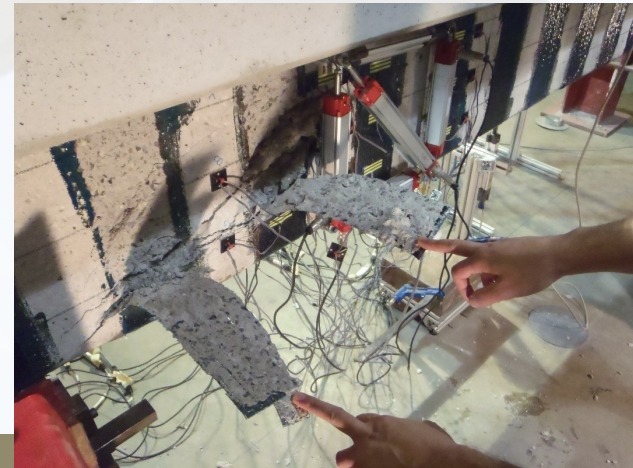


### 3. ULTIMATE LIMIT STATES

#### Shear



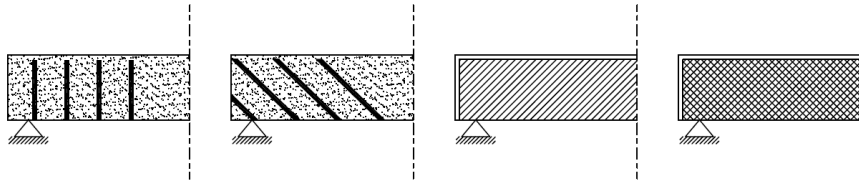
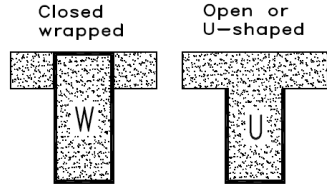
Problem: Debonding of FRP shear strengthening system



### 3. ULTIMATE LIMIT STATES

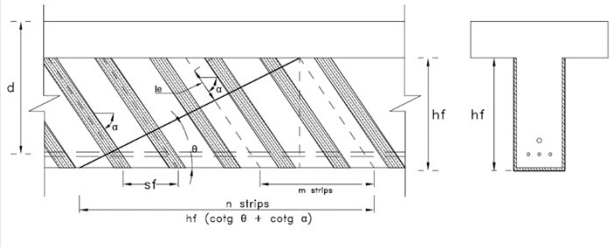
#### Shear

Configurations



Discontinuous

Continuous



$$\text{CASE 1 } f_{fwd} = \frac{2}{3} \cdot \frac{n \cdot s_f}{l_{bf,max,k} \cdot [(\cot\theta + \cot\alpha_f) \cdot \sin\alpha_f]} \cdot f_{bfRd}$$

$$\text{CASE 2 } f_{fwd} = \left[ 1 - \left( 1 - \frac{2}{3} \frac{m \cdot s_f}{l_{bf,max,k} \cdot [(\cot\theta + \cot\alpha_f) \cdot \sin\alpha_f]} \right) \frac{m}{n} \right] \cdot f_{bfRd}$$

Unstrengthened structure

$$\tau_{Rd,CFRP} = \tau_{Rd} + \tau_{Rd,f} \leq 0.5 \cdot v \cdot f_{cd}$$

$$\tau_{Rd,f} = \frac{A_f}{s_f} \cdot \frac{f_{fwd}}{b_w} \cdot (\cot\theta + \cot\alpha_f) \cdot \sin\alpha_f$$

$f_{fwd} = \text{????}$  FRP does not yield

**W Closed system**  $f_{fwd} = 0.8 \cdot k_r \cdot f_{fud}$

**U Open system**



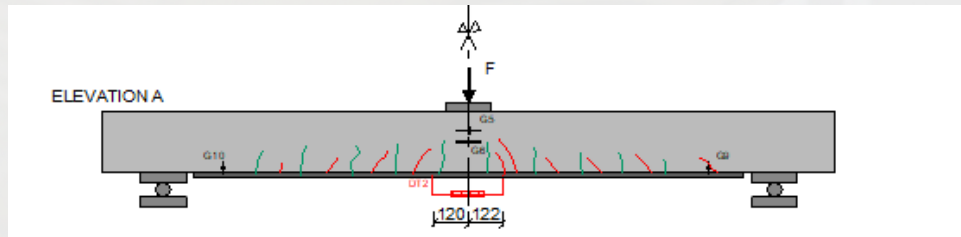
## 4. SERVICEABILITY LIMIT STATES

SLS (**control of deflections**) might govern the design of the strengthening system, even the main purpose was the strength increase.

Previous state of stresses and deflections should be considered in the verification of the SLS

$$\sigma_f \leq 0.8 \cdot f_{yk} \cdot \frac{E_f}{E_s} \quad \leftarrow \text{Stress limitations due to compatibility reasons}$$

**Cracking:**

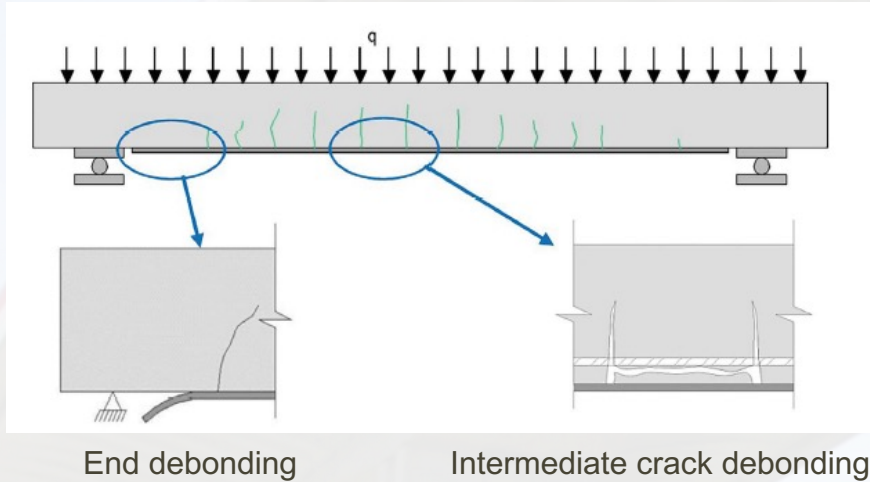


**Deflections:** Strengthened element should fulfil the deflection limitations given by the main EC2. Limited studies on the long-term behaviours.

## 6. BOND AND ANCHORAGE OF ADHESIVELY BONDED CFRP SYSTEMS

### Anchorage of EBR

#### Flexural strengthening



### End debonding

Effective bonded length  $l_{bf,max}$ : the minimum length that ensures the transfer of the maximum force or stress between the CFRP laminate and the concrete substrate.

- Refined method
- Simplified method

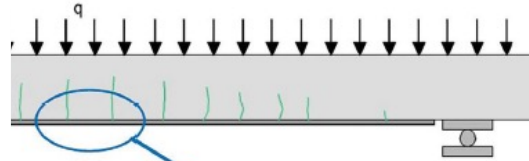
$$l_{bf,max,k} = 1.5 \sqrt{\frac{E_f \cdot t_f}{(f_{cm} \cdot f_{ctm,surf})^{0.5}}}$$

EBR shall be anchored from the section where the existing structure is able to carry the design load forces without any additional strengthening system

## 6. BOND AND ANCHORAGE OF ADHESIVELY BONDED CFRP SYSTEMS

### Anchorage of EBR

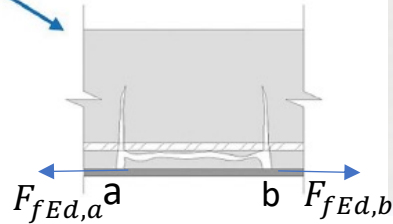
Flexural strengthening



$$\Delta f_{fEd} \leq \Delta f_{fRd}$$

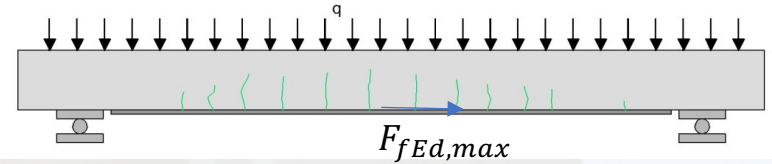
$$\Delta f_{fEd} = \frac{F_{fEd,b} - F_{fEd,a}}{b_f \cdot t_f}$$

$$\Delta f_{fRd} = \frac{1}{\gamma_{BA}} \cdot \left( (\eta_{cc} \cdot k_{tc} \cdot k_{tt})^{0.5} \cdot \Delta f_{fk,B} + \Delta f_{fk,F} + \Delta f_{fk,C} \right)$$



### Intermediate crack debonding

a) to limit the maximum CFRP strain or stress



b) to limit the increment of the tensile forces for each pair of adjacent cracks

Annex J is based on Finck and Zilch (2012) model included in the German guideline DAfStb (2013)





## CONCLUSIONS

**Strengthening existing concrete structures with CFRP adhesively bonded systems** has been incorporated for the **first time** in **EC2 provisions** in an informative annex (Annex J).

- Design provisions for strengthening existing reinforced or prestressed concrete structures in flexure, shear or confinement with passive EB or NSM CFRP reinforcements are given considering:
  - ✓ CFRP laminates are **linear elastic up** to failure.
  - ✓ **Debonding** is a **premature failure** that should be considered when strengthening in flexure and shear.



## ACKNOWLEDGEMENTS

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**Thank you for your attention**

Eva Oller